

A CONTINUOUS WATERSHED MODEL FOR EVALUATION AND
DESIGN OF FEEDLOT RUNOFF CONTROL SYSTEMS

by

Theodore A. Bean

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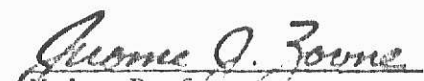
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INTRODUCTION

For the past two decades concern has grown in the U.S. over pollution of this nation's waters. Through the mid-sixties, the federal government formulated various general guidelines and provided financial assistance for antipollution programs, but encouraged the states to act on their own specific problems. As pollution levels increased through the late sixties, the federal government deemed it necessary to reorganize taking a firmer stance on the issue. Because of increased concern and the lack of pollution regulatory programs in some states, the U.S. Congress enacted the Federal Water Pollution Control Act Amendments of 1972 (Public Law 92-500) in October of 1972. The Act (FWPCA) required the establishment of effluent guidelines for various categories of polluters; one of which was the feedlot industry.

The national water discharge permit program was initiated on December 22, 1972 when regulations were promulgated and published in the Federal Register (24) establishing a new "Part 124 National Pollutant Discharge Elimination System (NPDES)" and "State Elements Necessary for Participation." On May 22, 1973, regulations establishing policies and procedures for issuance of NPDES permits were published (25) as Part 125. Subsequently, the Environmental Protection Agency (EPA) published an amendment (26) to this Part on July 5, 1973. The amendment provided for an expansion of exclusions which eliminated categories of small concentrated animal feeding operations and certain agricultural and silvicultural activities from the permit program. In promulgating the amendment, the EPA contended that while some point sources within the excluded categories should be regulated to be consistent with the purpose of the FWPCA, it would be difficult administratively if not impossible to issue individual

permits to all point sources. The Natural Resources Defense Council (NRDC) raised objection to this exercise of the EPA's discretion and challenged it in the Federal District Court of the District of Columbia in the case of "NRDC vs. Train." As a result of the court's ruling on June 10, 1975 to extend the NPDES permit program to include all point sources, the EPA proposed regulations (28) for applying the NPDES permit program to concentrated animal feeding operations on November 20, 1975.

Under the FWPCA, the EPA was charged to define and require the application of best practicable control technology currently available (BPT) to all existing facilities by July 1, 1977. Further, application of best available technology economically achievable (BAT) was required for all new and existing facilities by July, 1983. The effluent guidelines (27) published on February 14, 1974, in essence state "no discharge" except in the case of an extreme rainfall event. For the application of BPT, this event is the 10 year, 24 hour storm, while for application of BAT it is the 25 year, 24 hour storm.

As stated in the regulations of March 18, 1976 (29), feedlots are subject to case-by-case designation for evaluation under the NPDES permit program. This places a heavy burden on regulators who have to evaluate these facilities. A simple continuous watershed model originally developed by Koelliker (9) for evaluation of long term performance of runoff control facilities in Kansas and later used in other states has demonstrated usefulness as a design evaluation tool on the state level. Continuous watershed modeling involves the use of meteorological data as input to a computer program which simulates actual conditions occurring over a period of several years. Accordingly, a more complex computerized model which can appraise various management schemes of land disposal and

evaporation facilities at any location could be of great use to regulators for permit evaluation. Additionally, a model of this nature could be a valuable aid for both designers and enforcement agencies. The following describes such a model.

THE MODEL

The model evolved through selection of components from three different classes of existing models. These were combined to function simultaneously as one model. The first component is a model to generate runoff from the feedlot surface. The second is a storage facility model which accounts for pond level fluctuations in response to feedlot runoff inputs and irrigation disposal outputs. The third is a soil-moisture accounting model which enables the monitoring of conditions and the testing of disposal alternatives in the disposal area. Combined, these components provide a means to view interactions of various irrigation management schemes with daily meteorological factors.

A review of the existing models from which these components were taken clearly indicated the importance of establishing an accurate and flexible means to compute evaporation and evapotranspiration rates. Various methods for calculating daily evaporation and evapotranspiration were evaluated. The method used was selected primarily for its virtue of versatility in order to fulfill the requirements of any component of the model.

Throughout the evaluation and testing process emphasis was placed upon selection of physically based parameters while attempting to minimize inputs required. The ultimate goal was the development of a complete model from which any function could be analyzed in realistic physical

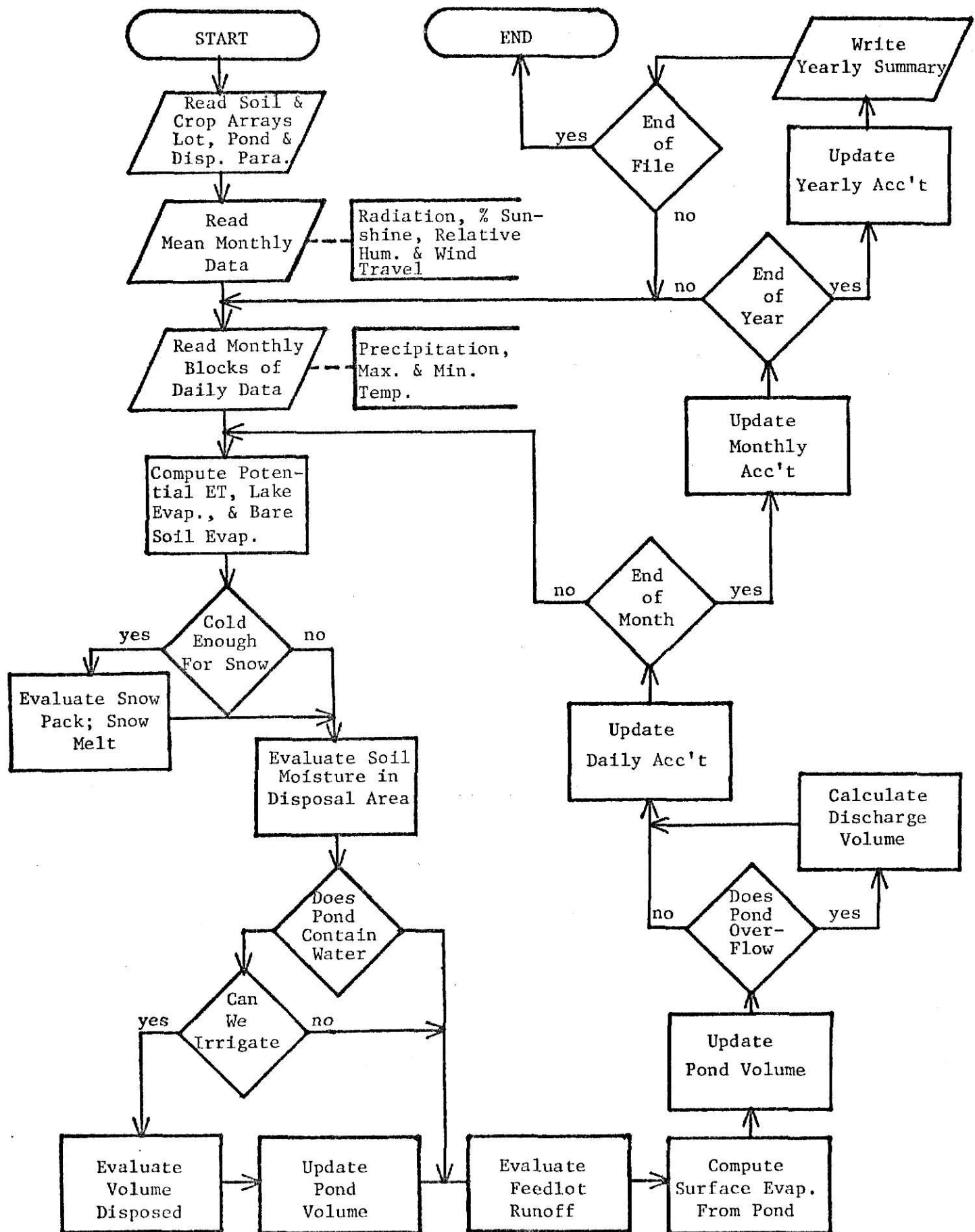
terms which would be readily adaptable to any geographic and climatic providence in the U.S. Achieving a realistic approach requires logical sequencing of operations. The general algorithm of the model is shown in Figure 1. The sequence of operations is an attempt to model the actual stream of events following a rainfall on the feedlot.

POTENTIAL EVAPOTRANSPIRATION

A number of temperature, radiation, humidity and combination methods for estimating potential evapotranspiration have been developed. The model requires a method which can be calibrated for use throughout the U.S. The method must not only give values which reflect differences in evaporation rates from the pond, bare soil, and vegetated disposal areas, but must also be applicable to the different climatic provinces of the country. In a recent study (2) fifteen methods were tested on four locations in Wyoming, Colorado, and Nevada. The Penman Combination method was ranked in the top five methods for estimating evapotranspiration. When calibrated and tested for Lake Hefner, Oklahoma (10), Kansas (34), Idaho and California (6), the Penman method produced reasonable results at all locations. The particular advantage of the method is its ability to estimate natural evaporation from water, bare soil, and vegetated areas. The original development is described by Penman (17) and an adaption of the method applied to irrigation scheduling is described by Jensen (6).

The method is a combination of the energy budget and mass transfer methods taking the form,

$$PET = \frac{\Delta}{\Delta + \gamma} (R_n - G) + \frac{\gamma}{\Delta + \gamma} (EA) \text{ ----- (1)}$$



where PET is the potential evapotranspiration, G is the soil heat flux, Rn is the daily heat budget at the surface, Δ is the slope of the saturation vapor pressure-temperature curve, γ is the psychrometric constant, and EA is a function of wind speed and humidity.

The soil heat flux (G) is a result of rapid changes in air temperature. During the summer months when day to day variation in temperature and radiation are not great, soil heat flux is small and can be neglected. Evapotranspiration is inhibited during the winter months and the effect of this term on ET is small. For these reasons, G can be neglected for purposes of this model.

The terms $(\frac{\Delta}{\Delta + \gamma})$ and $(\frac{\gamma}{\Delta + \gamma})$ are mean air temperature weighting factors having a sum equal to 1.0. As reported (34), they can be calculated by

$$\frac{\Delta}{\Delta + \gamma} = 0.039 (T_a)^{0.673} \text{ ----- (2)}$$

$$(\frac{\gamma}{\Delta + \gamma}) = 1.0 - \frac{\Delta}{\Delta + \gamma} \text{ ----- (3)}$$

to avoid the inconvenience of data tables and interpolation routines.

The heat budget term is developed in Gray (4, pp. 3.1-3.50) as follows:

$$R_n = (1.0 - r) R_{si} - R_b \text{ ----- (4)}$$

where Rsi is the total solar radiation incident to a water surface and

$$R_{si} = R_a (a + b \times \text{PSUNS}) \text{ ----- (4a)}$$

Then,

$$R_n = (1 - r) R_a (a + b \times \text{PSUNS}) \text{ ----- (5)}$$

where r is the mean daily shortwave reflectance or albedo, R_a is the extra-terrestrial solar radiation on a horizontal surface, $PSUNS$ is the percentage of possible sunshine, a and b are geographical constants, and R_b is the actual outgoing longwave radiation, where

$$R_b = P_e(0.98 - E)(0.1 + 0.9(PSUNS)) \text{ ----- (6)}$$

and,

$$P_e = \sigma(ABST)^4/58.1 \text{ ----- (7)}$$

where $ABST$ is the absolute temperature, σ is the Stefan-Boltzmann constant, E is the emissivity, and according to Brunt,

$$E = c + d\sqrt{ESA} \text{ ----- (7a)}$$

where ESA is the actual vapor pressure of the air, and

$$ESA = ES \times RHD \text{ ----- (8)}$$

where RHD is the mean relative humidity, c and d are geographical constants, and according to Linsley (14, p. 35),

$$ES = 33.9[(0.00738T+0.8072)^8 - 0.000019|1.8Ta+48|+0.001316] \text{ --- (9)}$$

where ES is the saturation pressure of air at the mean daily air temperature, and T_a is the mean daily air temperature.

The aerodynamic term in Eq. (1) is derived as follows:

$$EA = f(u)(ES - ESA) \text{ ----- (10)}$$

and

$$f(u) = 0.26 (e + 0.01 WVD) \text{ ----- (11)}$$

where $f(u)$ is a function of wind speed and

$$WVD = W(\log 6.6 / \log h) \text{ ----- (11b)}$$

where WVD is the mean wind velocity at 2 meters above the ground, W is the measured wind velocity, and h is the height above the ground at which the wind is measured,

Constants a, b, c and d in Eqns. (4a) and (7a) are listed by Gray (4, 3.9 & 3.19) for various locations. In Eq. (11) the constant e has a major effect on accuracy of the equation. However, e may be determined experimentally by setting the reflectance coefficient (r) equal to 0.05 (the albedo of a water surface) and adjusting the e value until the calculated PET equals mean annual lake evaporation for the location.

The complete, calibrated Penman Combination equation for Topeka, Kansas is

$$\begin{aligned} \text{PET} = & 0.039\text{Ta}^{0.673} [(1-r)\text{Ra} (0.22+0.54\text{PSUNS}) - 2.010 \times 10^{-9} (0.98-0.68 \\ & - 0.036\sqrt{\text{ES} \times \text{RHD}}) (0.1+0.9\text{PSUNS})] + (1-0.39\text{Ta}^{0.673}) \\ & \times 0.26(0.5+0.01\text{WVD}) (\text{ES}-\text{ES} \times \text{RHD}) \text{ ----- (12)} \end{aligned}$$

where a = 0.22, b = 0.54, c = 0.68, d = 0.036 and e = 0.5.

While an input value of r = 0.05 results in a potential equivalent to evaporation from a free water surface, the reflectance coefficients for green crops vary from 0.20 to 0.25 (4, p. 3.10). For this model, r = 0.23 will generally be used to calculate PET from the cropped disposal area while r = 0.20 will be used to calculate the potential evaporation rate from bare soil.

The remaining inputs to the Penman equation are solar radiation (Ra), relative humidity (RHD), wind travel (WVD), percent sunshine (PSUNS) and mean daily temperature (Ta). Although the Penman equation was determined to be the best available method for use in this model, it has the

disadvantage of requiring these daily inputs which are not readily available at all locations. In an attempt to sharply reduce model input requirements, Anschutz (personal communication) conducted a study to determine the effects of using mean monthly values of mean daily humidity, wind travel, solar radiation and percent sunshine. These means are readily available from all first order weather stations in the U.S. Using a differential t-test daily PET's calculated with daily inputs tested on a monthly basis were statistically different at the 95% confidence level from daily PET's based on monthly averages for five out of twelve months over a 10-year period. When the PET's were blocked and tested in five-day intervals, there were only two out of twelve months in which the two methods were statistically different at the 95% level. More importantly, when the mean monthly values were incorporated as inputs to the Kansas Watershed Model (34), an agricultural watershed model previously using daily inputs, no statistically significant differences were noted in daily streamflows. Even with the use of monthly averages, there are notable daily fluctuations in PET as a result of the variability of daily mean temperatures. Although these tests are indicative rather than conclusive, they represent sufficient evidence and justification for using monthly averages in this model. The increase in mobility and decrease of input data more than compensate for a slight decrease in daily accuracy.

WATER MOVEMENT WITHIN THE DISPOSAL AREA

The disposal area can be conceived as a soil-water reservoir which is recharged by precipitation and irrigation waters and depleted by evapotranspiration. Most annual row crops and grasses develop their root zone in the upper 120 cm. (4 ft.) of soil. Since soil water is extracted through the plant roots, most evapotranspiration occurs from

above this level. Root extraction pattern studies by Russell (19) and a consumptive use study by Manges (personal communication) conducted in England and Kansas have shown that approximately 70 percent is extracted from the upper 30 cm. (1 ft.) of soil, with nearly all of the remaining 30 percent being taken from the next 90 cm. (3 ft.). On this basis, the disposal area soil-moisture model simulates soil-water interactions above the 120 cm. (4 ft.) level by dividing the soil profile into two layers which are referred to as upper and lower zones, respectively. Evapotranspiration is proportioned in the two zones according to root densities, while evaporation from the soil surface is handled separately. Water which moves vertically downward through both zones is accounted as percolation out of the root zone.

Transfer of water from the soil to the atmosphere is handled by two processes; direct soil evaporation and transpiration. When soil lies fallow or annual row crops are in the early growth stage providing little vegetative cover, the transfer is accomplished by means of soil evaporation. As plant leaf surface areas increase through the growth stage, direct soil evaporation decreases and plant transpiration increases; hence, the transfer is termed evapotranspiration.

Soil Evaporation

Evaporation from soil generally occurs in two stages (18). First stage evaporation occurs when the soil is sufficiently wet to readily transport water to the surface. This is the constant rate stage in which evaporation proceeds at the potential rate calculated for bare soil. When a threshold amount, U , is reached, hydraulic properties of the soil begin to limit the evaporation rate. According to Ritchie (18), second

stage evaporation is calculated by

$$E_s = c't^{\frac{1}{2}} - c'(t-1)^{\frac{1}{2}} \text{ ----- (13)}$$

where E_s is the stage 2 evaporation, c' is a hydraulic coefficient of the soil, and t is the time after stage 1 evaporation.

Field work done by Kanemasu (8) using these relations have supplemented Ritchie's work, providing the values for U and c' shown in Table 1. Studies conducted by Bond (1) lead to similar relations using cumulative evaporation curves. The model treats both stage 1 and stage 2 evaporation in the same manner as Ritchie and Kanemasu.

Evapotranspiration

The actual rate of evapotranspiration (AET) is dependent upon atmospheric, plant, and soil factors (18). The atmospheric factors are incorporated in the computation of PET by the Penman method. The Blaney-Criddle method (21) is widely accepted in accounting for plant factors. This method is used to modify the potential rate by a plant consumptive use factor (k) which is determined from experimental data for each crop. Results of application to dry-land agricultural watersheds (34) and irrigated lands (6) display the versatility of the Blaney-Criddle method.

Under wet conditions evapotranspiration will proceed at the maximum rate. This relation continues until the soil-water deficit reaches a level which is equivalent to 0.3 of the maximum available moisture ($\theta_{\max}$) according to Kanemasu (8). Available moisture, or capillary water as defined by Israelsen (5, p. 162), is the moisture content of the soil between field capacity and the permanent wilting point. When the soil-water deficit falls below $0.3 \theta_{\max}$, Kanemasu modifies the

Irrigation Soil Class	SCS Soil Group	Soil Profile Depth Ft.	Available Water, inches of water		Field Capacity, inches of water		Permanent Wilting Point inches of water		Soil Moisture at Saturation inches of water		Upper Limit Stage 2 Evapor.	Empir- ical Coeff.
			Top 1' AVLFCU	1-4'* AVLFCU	Top 1' FCU	1-4'* FCL	Top 1' PWPUZ	1-4'* PWPLZ	Top 1' SMSATU	1-4'* SMSATL	mm U	mm day C
1	D	3	2.6	2.7	4.6	9.4	2.0	6.7	5.8	11.8	12	5.08
2	D	3	1.5	2.9	4.4	9.4	2.9	6.5	6.2	11.5	12	5.08
3	C	5	2.5	5.7	4.5	14.2	2.0	8.5	5.7	16.3	10	4.5
4	C	2.5	2.4	2.5	4.6	7.0	2.2	4.5	5.7	8.2	10	4.5
5	B	5	2.5	6.7	4.5	13.9	2.0	7.2	5.7	15.8	10	4.5
6	B	3	2.6	4.2	4.3	9.1	1.7	4.9	5.5	10.4	10	4.5
7	B	5	2.4	6.6	4.0	13.7	1.6	7.1	5.4	15.4	9	4.04
8	B	2.5	2.4	3.3	4.0	6.8	1.6	3.5	5.4	7.7	9	4.04
9	B	5	2.4	5.2	3.8	9.2	1.4	4.0	5.3	13.9	8	3.5
10	B	5	2.2	4.1	3.5	7.0	1.3	2.9	5.2	13.2	8	3.5
11	B	5	1.5	4.1	2.3	7.0	0.8	2.9	4.8	13.2	7	3.4
12	A	5	1.0	2.5	1.7	4.3	0.7	1.8	4.8	12.9	6	3.34

TABLE 1 - Properties Used In The Model

*Lower zone depth is taken as 3' except where soil profile depth limitations occur.

potential rate by the simple linear relation

$$K_s = \theta_a / 0.3 \theta_{\max} \text{ ----- (14)}$$

where θ_a is the actual available soil moisture content. With the application of K_s to the potential rate, the soil-water deficit allows AET to be at the maximum rate above $0.3 \theta_{\max}$ and limits it to zero at the permanent wilting point. The resulting relation for the actual rate of evapotranspiration is

$$AET = PET \times k \times K_s \text{ ----- (15)}$$

Surface Runoff

The Soil Conservation Service method (30) is widely accepted for predicting the total volume of surface runoff produced by a design storm. The equation takes the form

$$Q = \frac{(P - 0.2S)^2}{P + 0.8S} \text{ ----- (16)}$$

where Q is direct surface runoff, P is precipitation, and S is the maximum potential difference between precipitation and runoff. The initial abstraction, $IA = 0.2S$, which consists of surface storage, interception losses, and water which infiltrates into the soil prior to runoff, must be satisfied before surface runoff can occur.

Development of the SCS method entailed assigning arbitrary runoff curve numbers to evaluated antecedent moisture, soil conditions, land use and conservation practices. For user convenience input runoff curve numbers (N) shown in Table 2 are specified by input soil (32) and crop types defined in Table 3 and Table 4, respectively. The input N (30) is based on condition II antecedent moisture, which is between 0.5 and 0.8 of field capacity (8,18) for the specified soil during the growing season or between 0.6 and 0.9 of field capacity during the dormant season. When the soil moisture is less than 0.5 or 0.6 of field capacity,

Soil Class	Row Crops	Alfalfa	Wheat	Pasture	Fallow
1	86	83	84	80	84
2	86	83	84	80	84
3	82	78	81	74	78
4	82	78	81	74	78
5	75	69	73	61	69
6	75	69	73	61	69
7	75	69	73	61	69
8	75	69	73	61	69
9	75	69	73	61	69
10	75	69	73	61	69
11	75	69	73	61	69
12	65	55	61	39	61

TABLE 2 - SCS Runoff Curve Numbers for Condition II*

*Condition II - During the growing season, soil moisture in the top 1' is between 0.5 and 0.8 of field capacity. For the non-growing season, the range is 0.6 to 0.9 of field capacity.

Irrigation Soil Class	Profile Depth (ft.)	Soil Class Description
1	3'	Deep soils with silt loam or silty clay loam surface layers and slowly to very slowly permeable heavy clay and claypan subsoils.
2	3'	Deep soils with silty clay or clay textures throughout. Surface infiltration and subsoil permeability are very slow when the soil is moist. Shrinkage from drying causes extensive cracking, resulting in high infiltration rates until swelling occurs.
3	5'	Deep soils with silt loam, loam, clay loam, or silty clay loam surface layers and clay loam, silty clay loam, or silty clay subsoils. Subsoil permeability is slow to moderately slow. Shrinkage cracks resulting from drying in the soils with more clayey subsoil textures give a relatively high initial infiltration rate.
4	2.5'	Moderately deep soils with silt loam, clay loam, or silty clay loam surface layers and clay loam or silty clay subsoils with predominately moderately slow permeability.
5	5'	Deep soils with silt loam, loam, clay loam, or silty clay loam surface layers and subsoils. Subsoil perm: moderate to moderately slow.
6	3'	Moderately deep soils with silt loam or loam surface layers and loam, clay loam, or silty clay loam subsoils with moderate to moderately slow permeability.
7	5'	Deep soils with silt loam, loam or very fine sandy loam surface layers and moderately permeable, medium textured subsoils.
8	2.5'	Moderately deep soils with silt loam, loam or very fine sandy loam surface layers and moderately permeable clay loam, loam, or silt loam subsoils.
9	5'	Deep soils with fine sandy loam and loam surface layers and subsoils that have moderatley rapid permeability. Available water capacity is moderate to low.
10	5'	Soils are moderately deep over sand with sandy loam to loam surface layers and moderately rapid to rapidly permeable subsoils with low available water capacity.
11	5'	Deep soils with loamy fine sand or loamy sand surface layers and moderately rapid to rapidly permeable subsoils
12	5'	Deep rapidly permeable soils with sand or fine sand textures throughout.

TABLE 3 - Irrigation Design Class Descriptions for Soils in the Disposal Area

<u>Reference Number</u>	<u>Crop</u>
1	Wheat
2	Sorghum
3	Corn
4	Soy Beans
5	Pasture
6	Alfalfa
7	Fallow Soil

TABLE 4 - Crop Types

dependent upon the season, N is modified to a condition I antecedent soil moisture by

$$N_I = N \times 0.39e^{(-0.009xN)} \text{ ----- (17)}$$

When the soil moisture is greater than the upper limit set for the season, the condition III curve number is obtained by the equation

$$N_{III} = N \times 1.95e^{(-0.00663xN)} \text{ ----- (17a)}$$

Then by definition

$$S = \frac{1000}{N} - 10 \text{ ----- (18)}$$

and S is substituted into Eq. (16) to calculate the runoff volume. The use of Eqns. (17) and (17a) to modify N alleviates the inefficiencies associated with look-up tables and interpolation schemes in computer simulation.

Interception

After calculation of surface runoff, interception losses must be separated from the remainder of the initial abstraction. Many equations exist which take into account variations of evaporation rates, vegetation canopy, rainfall intensity and total event volume. Use of one of these methods would require many complex inputs which are unnecessary for other functions in the model. A less complex approach for computer simulations (20) (14, p. 323) requires fixing an interception-storage capacity which must first be satisfied before water from any event is available for infiltration. For this model, the interception-storage was fixed at 2.54 mm (0.1 in.) and depleted at the potential rate. According

to Ward (32), this storage should account for from 10 to 20 percent of the annual precipitation. Of the previously tested Kansas and Oregon stations approximately 16 to 18 percent of the annual precipitation was accounted as interception-storage losses which evaporated back into the atmosphere.

Infiltration and Redistribution

Water infiltrates at a higher rate at the beginning of a storm. The infiltration rate and the redistribution of water within the soil profile is dependent upon the hydraulic properties of the soil (5, p. 152). Actual rates are difficult to simulate because of the variability of influencing factors. Saxton (20) has modeled infiltration by means of physical redistribution. Using this scheme, a soil layer is allowed to take on an equivalent amount of water to raise storage to a level of 0.9 saturation with any excess amount being cascaded to the next successive layer. This continues until all water available for infiltration is stored. Movement of gravitational water is then computed by the one-dimensional Darcy equation for unsaturated flow using moisture-tension and moisture-conductivity relationships.

Redistribution is approached in a similar manner by this model with some simplifying modifications. Since field capacity is generally evaluated two days after saturation (5, p. 163) and relationships required for use of the Darcy equation are difficult to program, the model allows soil moisture to decrease to field capacity after two days if it is not already below that level as a result of AET needs. This is accomplished by setting the soil moisture equal to field capacity and redistributing the excess to the next zone. Since the lower zone generally does not receive water as a result of small events, greater periods of time pass

between recharges. When left undisturbed for greater periods of time, soils will gravitationally achieve a soil-moisture level which is less than field capacity. Accordingly, the lower zone soil moisture is decreased to 0.9 of field capacity when the time between recharges exceeds two days and the excess is considered to percolate out of the root zone. This modified method appears to result in reasonable estimates of vertical movement of water for the soil groups tested.

Snow on the Disposal Area

At many locations in the U.S., a sizable snow pack accumulates during the winter months. For this reason, a rational method of accumulating and dissipating snow must be incorporated to approximate actual conditions on the disposal area. Numerous snowmelt methods exist which are based on thermodynamic principles, empiricisms, and degree-day factors. Approaches such as presented by Leaf (12) based on thermodynamic principles are good representations of actual snow physics. The primary disadvantage of this type of approach is that it requires many inputs. For this reason a thermodynamic approach is considered too rigorous for use in this general model. Empiricisms developed to approximate the thermodynamic principles are simpler but generally provide less satisfactory results (14, p. 275). The degree-day approach stands on the virtue of simplicity in requiring only dry-bulb temperature as input. The method has the disadvantage of neglecting humidity, radiation, and other factors which influence snow melt. In spite of this, good results have been obtained (13) using a degree-day approach.

For purposes of this model, precipitation events occurring on days having an average temperature $\leq 0^{\circ}\text{C}$ (32°F) were accumulated as water

equivalents of snow. The snow melt function used in this model as presented by Gray (4, pp. 9.12-9.14) is

$$M = C(T_a - T_b) \text{ ----- (19)}$$

where M is the snow melt, T_a is the mean daily atmospheric temperature, T_b is the base temperature, and C is the degree-day coefficient. This computes snow melt as a result of atmospheric conditions. If the snow is initially considered to be at 0°C (32°F), then the relation used by Linsley (13) to compute snow melt by rainfall is

$$D = (1/144)(P)(T_a - 32) \text{ ----- (20)}$$

where D is the snow melted by rainfall, P is the amount of rainfall, and T_a is the mean daily temperature. The heat of fusion of ice is 79 cal./g. (144 BTU/lb.) while the specific heat of water is 1 cal./g. (1 BTU/lb.).

Evaporation from snow (sublimation) works basically on the same principles as evaporation from a free water surface (4, pp. 3.7-3.8). Approximately 5 percent of incident shortwave radiation is reflected from a free water surface while from 80 to 90 percent is reflected by a clean, dry snow surface. Because of changes in crystalline structure, density, and the amount of dirt on the surface, the albedo drops to 50 percent or less as snow ages. Accordingly, an average albedo (r) of 70 percent has been chosen for application of the Penman equation to a snow surface in this model. Additionally, it has been assumed that a water equivalent of 2.54 mm. (0.1 in.) in the pack provides sufficient cover to treat the evaporation calculation in terms of a snow surface. This is accomplished with ease by applying a value of $r = 0.70$ to the Penman method when the pack contains a water equivalent in excess of 2.54 mm. (0.1 in.). The

resulting daily sublimation from the pack is small, generally less than 0.25 mm. (0.01 in.), but through the winter months this may account for noticeable dissipation of the pack.

Criteria for Disposal on the Area

Water from the storage facility will not be applied on the disposal area under the following conditions:

- 1) when there is < one full day's irrigation water in the facility;
- 2) when the ground is frozen;
- 3) when the mean daily temperature < 0°C (32°F), and
- 4) when the upper zone soil moisture is greater than a specified percentage of field capacity.

An additional condition that could be easily added is one which does not allow irrigation when the crop on the area is not in its growing stage. The design of the model allows for simple modification to accommodate almost any irrigation management scheme. The soil is considered to be frozen if the sum of the previous two days temperature < 0°C (64°F), i.e., the 2-day average temperature < 0°C (32°F). Thawing is considered to occur when the sum of the average temperatures for any three consecutive day period is > 10°C (114°F), i.e., the 3-day average temperature > 3°C (38°F).

The percentage of upper zone field capacity (PSMN), which defines the third condition for irrigation is an input variable which allows various irrigation management schemes to be tested. By setting PSMN = 0.0, a non-irrigation or pure surface evaporation system can be tested.

FEEDLOT FUNOFF

Feedlot surfaces greatly differ from disposal area surfaces. Repeated animal milling works the surface, compacting underlying soil layers. As

a result, normal infiltration is impaired greatly and largely influenced by the development of the lot. Difficulties therefore arise when attempting to apply normal soil-moisture models to a feedlot.

Models incorporating the SCS method to predict feedlot runoff volume have been tested in Kansas (9), Oregon (33), and Minnesota (11). Reasonable results were obtained by using curve number (N) varying from 91 to 98 depending on surface type and antecedent conditions. General agreement exists that for antecedent conditions I and II, $N = 91$ is satisfactory while $N = 97$ works well for condition III antecedent moisture on unsurfaced lots.

During extensive testing with feedlot runoff data from feedlots at Gretna, Nebraska (23), Bushland, Texas (3), and Pratt, Kansas (15), the SCS method was evaluated along with numerous modifications. Attempts to evaluate runoff curve numbers by means of modeling the soil-moisture deficit of the feedlot surface provided less satisfactory results than a much simpler model used by Koelliker (9). Studies by Miner (16) and Swanson (23) suggest that rainfalls of certain intensity and duration are required before runoff occurs from the feedlot surface. Accordingly, variations of antecedent moisture required to produce runoff were tested. It was discovered that calculated runoff quantities resulting from large rainfall events were underestimations of actual runoff quantities. Since infiltration is greatly impeded in a developed lot, it was assumed that above a certain maximum amount of water required to saturate the soil above the impervious layer and fill depression storage, any additional precipitation would run directly off. Various event magnitudes were tested to evaluate the maximum storm occurring before any excess essentially all runs off the feedlot surface.

The model which eventually provided the best results specifies that surface runoff will not occur if

- 1) the 3-day antecedent moisture and the precipitation on the day of concern is < 1.3 cm. (0.5 in.);
- 2) the average temperature for the day $< -1^{\circ}\text{C}$ (30°F);
- 3) the soil is frozen.

While the ground is frozen, all precipitation is accumulated as snow on the feedlot. When the soil thaws, the accumulated volume of snow is allowed to run off with $N = 97$. Snow on the feedlot is handled differently than on the disposal area because of major differences in the two surfaces. In most parts of the U.S., a volume nearly equivalent to the accumulated winter precipitation quantity is retained in the storage facility during the course of the season. Even at a high N , routing small melts during winter months through the SCS equation would result in a significantly smaller volume. Urine addition by confined animals compensates for sublimation losses from snow and may account for differences discussed. Irrigation conditions are generally unfavorable in winter so the timing of additions to the pond is not as important as the volume accumulated during the season.

Best results are obtained by partitioning the year into a growing season and dormant season; April-October and November-March, respectively. During the growing season, a maximum of 3.18 cm. (1.25 in.) is routed through the SCS equation with $N = 97$ if the 3-day antecedent moisture is > 1.91 cm. (0.75 in.) or $N = 91$ if the antecedent moisture $\leq$ this amount. For any day having an event > 3.18 cm. (1.25 in.), the excess is considered to run directly off the feedlot surface. During the dormant season, the maximum precipitation routed through the SCS equation is lowered to 2.54 cm. (1.0 in.) and $N = 91$ is used when 3-day antecedent

moisture < 1.27 cm. (0.5 in.). Otherwise, $N = 97$ is used during this season. Calculated runoffs exceeding 0.06 in. are then reduced by that amount.

A regression analysis between calculated values of feedlot runoff and actual data resulted in a normal range of r^2 values between 0.80 and 0.85 on unsurfaced lots. At the present time no attempts to simulate surfaced lots have been made since there is a lack of actual data for this type of lot. The regression analysis on unsurfaced lot data appears to justify the procedures described above for modeling the feedlot runoff.

THE STORAGE FACILITY

A runoff control structure generally resembles an inverted frustrum of a pyramid. For this reason the configuration of the general prismatoid (Figure 2), which allows independent variations of base length (L), base width (W), and side slopes (S), was used in the model. These parameters along with the maximum depth of water (HMAX) are input variables to the program and define the maximum volume of water which the facility can contain before a discharge occurs.

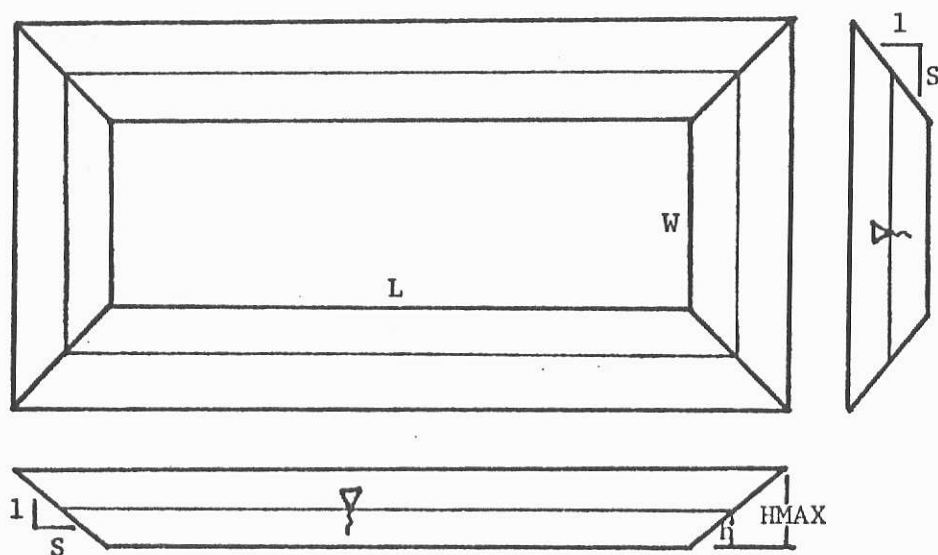


FIGURE 2 - Configuration of General Prismatoid

By specifying a particular size and shape of pond, the surface area can be computed for any storage volume. The volumetric equation for a general prismatoid (22) is

$$V = \frac{1}{6} h (B_1 + 4B_m + B_2) \text{ ----- (21)}$$

where V is the volume, h is the depth, B₁ is the bottom surface area, B₂ is the top surface area, and B_m is the area of a plane at h/2 above the bottom.

Feedlot runoff contains a high suspended solids load which impedes seepage from the storage facility. Conservation and irrigation ponds containing water which does not have large concentrations of suspended solids tend to seal over a period of time. Although seepage may continue for certain soils and for facilities which experience wet/dry cycles, no verified exfiltration rates are available. For this reason, a sealed condition or zero exfiltration is assumed for the pond. The inputs to the facility include runoff from the feedlot and direct contributions of precipitation to the receiving area. Outflows consist of water pumped out for irrigation purposes, surface evaporation, and overflows.

Pond Volume Accounting Hierarchy

In budgeting the inputs and outputs of the storage facility, the volume is the basic quantity which varies in response to climate and disposal requirements. Although it is probably of no particular consequence in terms of final results, the order in which the accounting proceeds is as follows:

- 1) The calculated volume of water disposed for the present day is deducted from the volume of water remaining in the pond at the end of the previous day.
- 2) Surface evaporation is deducted. The calculation of surface evaporation is accomplished by using Eq. (21) and Newton's

Method of Successive Approximations to evaluate the depth of water (h). Direct solution for h involves the solution of a cubic equation which, as a result of imaginary roots, may create difficulties with a computer solution. The water surface area (B_2) is then calculated and multiplied by lake evaporation (from the Penman equation) to obtain the surface evaporation. The pond is considered to be frozen when the soil is frozen and no surface evaporation occurs when the pond is frozen.

- 3) The feedlot runoff volume calculated for the day and the precipitation falling directly on the pond receiving area is added to the volume in the pond. The direct receiving area is the maximum surface area for h equal to H_{MAX} .
- 4) The calculated volume is compared to the maximum volume of the facility (VOL_{MAX}). If the volume exceeds VOL_{MAX} , the excess is accounted as discharge and the pond volume is set equal to VOL_{MAX} . Either the calculated volume or VOL_{MAX} , then, is the volume of water remaining at the end of the day.

DISCUSSION AND RESULTS OF PRELIMINARY TESTING

The model has been thoroughly tested with data from Topeka, Kansas and has been run for other Kansas and Oregon stations. Required inputs of solar radiation, relative humidity, percent sunshine, and wind speed were taken as mean monthly daily averages from meteorological data. Daily inputs of precipitation and maximum and minimum temperature were obtained off data tapes purchased from the National Weather Service, Asheville, North Carolina. Presently, the model is capable of reading data from any 9-track tape prepared with Asheville format. Storage facility sizes tested were equivalent to 1x, 2x, and 3x multiples of the volume accumulated from the 25-year, 24 hour storm, provided all that fell on the feedlot surface ran off.

At Topeka, Kansas the 1x facility proved to be very insufficient allowing 29 discharges during the 25-year period between 1949 and 1973. The 2x facility provided better control by decreasing the number of

discharges occurring during this period to 13. As was expected, the 3x facility was sufficient to hold the accumulated process waters having no discharges during this period. Using the management scheme of irrigating when the soil moisture in the disposal area is below 90 percent of field capacity, a maximum of 82.5 percent of the 3x facility volume was required. During this period the average precipitation was 34.3 inches causing an average runoff of 13.0 inches from the feedlot surface. This was accumulated over a 40 acre feedlot and disposal was on an 80 acre area having sorghum as the growing crop. Less than one minute of computer execution time was required to process the 25 years of data.

During the next year, the model will be calibrated and tested at locations throughout the U.S. The capabilities and versatility of the model will be examined during this time. At present, the model specifies only one irrigation disposal area. In the future, the model will be modified to enable simulation of irrigation on various portions of the area at one time. This will allow a wider variety of irrigation practices to be evaluated.

SUMMARY

Increasing levels of water pollution and public concern have prompted the formation and imposition of federal regulations in an attempt to alleviate sources of pollution. Implementation of the NPDES program and provisions for application to all point sources including concentrated animal feeding operations have placed a heavy burden on administration and enforcement agencies. On the state level, simple continuous watershed models have proven useful as design aids and design

evaluation instruments for storage facilities. Accordingly, a model which can analyze various management schemes and evaluate the full perspective of land disposal and evaporation techniques is a necessary tool not only for enforcement agencies, but for the feedlot operators as well.

A physically based model which can readily evaluate most any land disposal or evaporation scheme has been developed. The model has the capability of being applied at any location in the U.S. with only minor calibration. Input data requirements include meteorological factors which are readily available for all parts of the country. Preliminary testing has demonstrated the capabilities and accuracy of the model. Specific systems can easily be programmed and evaluated utilizing a minimum of computer time.

**THIS BOOK
CONTAINS
NUMEROUS PAGES
WITH MULTIPLE
PENCIL AND/OR
PEN MARKS
THROUGHOUT THE
TEXT.**

**THIS IS THE BEST
IMAGE AVAILABLE.**

APPENDIX I - References

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APPENDIX II - Notation

The following symbols are used in this paper:

a,b,c,d & e = geographical constants for use in the Penman equation

ABST = absolute air temperature, in degrees Kelvin

B_m = area of the midsection of the storage facility, in square feet

B₁ = area of the bottom of the storage facility, in square feet

B₂ = the water surface area in storage facility, in square feet

C = degree-day coefficient (usually 0.05-0.06), in inches per day - degree Fahrenheit

C' = hydraulic coefficient of soil

D = snow melted by rainfall, in inches of water

E = emissivity

E_a = convective loss, in millimeters of water evaporated per day

E_s = saturation vapor pressure, in millibars

ESA = actual vapor pressure of the air in millibars

E_s = stage 2 evaporation, in inches of water per day

G = sensible heat flux to or from the soil

h = depth of water in the storage facility, in feet

HMAX = maximum depth of water allowable in the storage facility, in feet

I_A = initial abstraction, in inches of water

k = Blaney-Criddle consumptive use coefficient

K_s = coefficient for soil-moisture conditions

M = atmospheric snow melt, in inches of water

N = SCS runoff curve number

P = precipitation, in inches

- PET = potential evaporation, in millimeters of water
- PSUNS = percentage of possible sunshine
- Q = direct surface runoff, in inches
- r = mean daily shortwave reflectance or albedo
- Ra = extra-terrestrial solar radiation on a horizontal surface, in millimeters of water evaporated per day
- Rb = actual outgoing longwave radiation, in millimeters of water per day
- Rn = heat at the surface, in millimeters of water per day
- Rsi = total solar radiation incident to a water surface, in millimeters of water evaporated per day
- S = maximum potential difference between precipitation and runoff, in inches
- t = time after stage 1 evaporation, in days
- Ta = mean daily temperature, in degrees Fahrenheit
- Tb = base temperature, in degrees Fahrenheit
- V = volume of water in the storage facility, in cubic feet
- VOLMAX = maximum volume of water held by the storage facility, in cubic feet
- W = measured wind speed, in miles per day
- WVD = mean wind speed at 2 meters above the ground, in miles per day
- γ = psychrometric constant in the Bowen ratio equation
- Δ = slope of the saturation vapor pressure-temperature curve
- θ_a = actual available soil moisture content, in inches of water
- $\theta_{\max}$ = maximum available soil moisture content, in inches of water
- σ = Stefan-Boltzman constant (2.01×10^{-9} mm/day °K)

APPENDIX III
Computer Printouts

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MAIN

FORTAN IV G LEVEL 21

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C      FRCS - VERSION 1
C      JOINTLY PROGRAMMED BY THE CIVIL AND AGRICULTURAL ENGG. DEPT.
C      KANSAS STATE UNIVERSITY
C      MARCH 1976
C
C001  INTEGER CRCP,CAY,FROZE,SGIL,STINC,T,YEAR,YEARS,YEND,YSTART
C002  REAL IA,IAADD,IAET,KRCRCF,KS,LAKEVP,LKEVPT,LTAREA,L,M,MA
C003  REAL MAXVCL,M
C004  DIMENSION APCNTH(13),AVLFCL(12),AVLFCL(12),C(12),FCL(12),FCU(12)
C005  DIMENSION KRCRPT(6),NDI(12),PCACCT(13,8),PREC(31),PSUNS(12)
C006  DIMENSION PURLZ(12),PURLZ(12),RA(12),RCM(12,7),RCN(12,7),RHD(12)
C007  DIMENSION SWACCT(13,8),SPSATL(12),SPSATL(12),TAGV(13),TMAX(31)
C008  DIMENSION TWIN(31),U(12),WIND(12)
C009  COMPLEX*16 KRCPC(16(6)),WHEAT*,SORGHUM*,CORN*,SCY BEANS*,
C010  PASTURE*,ALFALEA*,
C011  DATA APCNTH,JAN,FEB,MAR,APR,MAY,JUNE,JULY,
C012  AUG,SEP,OCT,NOV,DEC,TOT,
C013  DATA AVLFCL,2.6,1.5,2.5,2.4,2.5,2.6,2.4,2.4,2.4,2.2,1.5,1.0,
C014  DATA C,2.2,2.2,2.2,2.2,2.2,2.2,2.2,2.2,2.2,2.2,2.2,2.2,
C015  DATA C,134,C,131/
C016  DATA FCU,4.4,4.4,4.4,4.4,4.4,4.4,4.4,4.4,4.4,4.4,4.4,4.4,
C017  DATA FCL,5.4,9.4,14.2,7.0,13.5,9.1,13.7,6.8,9.2,7.0,7.0,4.3/
C018  DATA NDI,31,28,31,20,31,20,31,30,31,30,31,30,31/
C019  DATA PURLZ,2.0,2.0,2.0,2.0,2.0,2.0,2.0,2.0,2.0,2.0,2.0,2.0,
C020  DATA PURLZ,6.7,6.5,8.5,4.5,7.2,4.5,7.1,3.5,4.0,2.9,2.9,1.8/
C021  DATA SPSATL,5.8,6.2,5.7,5.7,5.7,5.5,5.4,5.4,5.4,5.4,5.4,
C022  DATA SPSATL,11.8,11.5,16.3,16.2,15.8,10.4,15.4,7.7,13.9,13.2,13.2,
C023  DATA U,0.47,0.47,0.39,0.39,0.39,0.39,0.39,0.35,0.35,0.31,0.28,
C024  DATA C,24/
C025  DATA AET,AETLZ,AETLZ,2,DISVOL,OPERC,EO,EXCESS,IA,IAADD,IAET,M,MA,OC13C90
C026  DATA PCACCT,PACKPY,PCVCL,PERC,PCNVL,PL,P2,P3,SACMLT,TOPERC,TIME,
C027  DATA CRCP,CAY,ICAY,SCIL,T/5*0/
C028  DATA SWLZ,SPMAXL,SMUZ/9.35,2.2,3.25/
C029  NAMELIST/ALPHA/CROP,OCRP,GRCP,POT,PSMN,RCROP,SOIL
C030  NAMELIST/BETA/DSAREA,DSRATE,HMAX,INDST,L,LTAREA,MSTART,S,STORM,W,
C031  *YEND,YSTART
C
C      ***** INPLIS *****
C
C032  READ THE SOIL AND CRCP ARRAYS
C033  READ(5,20) NAME,OF,CITY,AND,STATE
C034  20 FORMAT(20X,5A4)
C035  READ(5,40) ((KRCROP(I,J),J=1,12),I=1,6)
C036  40 FORMAT(12F4,2)
C037  READ(5,60) ((RCN(I,K),K=1,7),I=1,12)
C038  60 FORMAT(17F2,C,1X)
C039  READ THE LCT,PCNC AND DISCOSAL PARAMETERS
C040  READ(5,ALPHA)
C041  READ(5,BETA)
C042  READ THE MONTHLY AVERAGE METEOROLOGICAL DATA
C043  READ(5,80) (PSUNS(I),RHC(I),RA(I),WIND(I),I=1,12)
C044  80 FORMAT(12X,F2.2,F2.0,F4.2,F3.1)
C
C045  A1,A2,A3,A4,AND A5 ARE COEFFICIENTS USED IN VOLUME ROUTINE

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0148 IA=0.0 0032000
0149 GC TC 640 0033000
0150 600 RNOF=C.0 0033400
0151 620 IA=0.1 0033500
0152 IF(IA.GT.RAIN) IA=RAIN 0033600
0153 IF((IA+IAACC).GE.C.1) IA=C.1-IAADD 0033700
0154 EVALUATE PERCOLATION INTO UPPER ZONE 0033800
0155 640 PERC=RAIN-RNOF-IA 0033900
0156 UZEVAP=C.0 0034000
0157 CALCULATE PRESENT STORAGE AVAILABLE IN UPPER ZONE 0034100
0158 SPMAXL=0.9*SMSTAU(SCIL)-SPUZ 0034200
0159 EVALUATE WATER CASCADED TO LOWER ZONE FOR STORAGE 0034300
0160 PERCL=PERC-SPMAXU 0034400
0161 IF(PERCL.LT.0.0) PERCL=C.0 0034500
0162 IF (SMUZ.GT.FCL(SOIL)) GO TC 660 0034600
0163 EXCESS=0.0 0034700
0164 GC TO 680 0034800
0165 EVALUATE GRAVITATIONAL WATER IN UPPER ZONE 0034900
0166 660 EXCESS=SMUZ-FCL(SCIL) 0035000
0167 IF THE CROP IS DORMANT OR THE SOIL LIES FALLOW, SCIL 0035100
0168 C*** EVAPORATION IS EVALUATED 0035200
0169 680 IF(KCRCPC(CPCP,M),LE.0.0) GC TO 860 0035300
0170 C*** CHECK WHETHER SOIL MOISTURE LIMITS AET FROM THE UPPER ZONE 0035400
0171 IF(SMLZ-10.3*(AVLFCU(SCIL))+PMUZ(SCIL))700.70C.760 0035500
0172 CALCULATE AET FROM THE UPPER ZONE WHEN LIMITED BY SOIL MOISTURE 0035600
0173 700 AVAILU=SMUZ-PMUZ(SCIL) 0035700
0174 IF(AVAILU.LE.0.0) AVAILU=C.0 0035800
0175 AETUZ=0.7*AET*(AVAILU/C.3*AVLFCU(SCIL))) 0035900
0176 EVALUATE AVAILABLE WATER IN THE LOWER ZONE 0036000
0177 AVAILL=SMUZ-PMPLZ(SCIL) 0036100
0178 IF(AVAILL.LE.0.0) AVAILL=C.0 0036200
0179 C*** CHECK WHETHER SOIL MOISTURE LIMITS AET FROM THE LOWER ZONE 0036300
0180 IF(SMLZ-(C.3*(AVLFCU(SCIL))+PMPLZ(SCIL))) 720.720.740 0036400
0181 CALCULATE AET FROM THE LOWER ZONE WHEN LIMITED BY SOIL MOISTURE 0036500
0182 720 AETLZ=0.3*AET*(AVAILL/C.3*AVLFCU(SCIL))) 0036600
0183 GC TC 78C 0036700
0184 740 AETLZ=AET-AETUZ 0036800
0185 GC TO 780 0036900
0186 C*** EVALUATE AET FROM BOTH ZONES UNDER WET CONDITIONS 0037000
0187 760 AETUZ=0.7*AET 0037100
0188 AETLZ=0.3*AET 0037200
0189 AVAILL=SMUZ-PMPLZ(SCIL) 0037300
0190 IF(SMLZ.LE.C.3*(AVLFCU(SCIL))+PMPLZ(SCIL)) 80 TC 720 0037400
0191 C*** EVALUATE SOIL MOISTURE 0037500
0192 800 SPMUZ=SMUZ+PERC-AETUZ-EXCESS 0037600
0193 SMLZ=SMUZ-AETLZ+EXCESS 0037700
0194 GC TC 560 0037800
0195 820 SMUZ=SMUZ+SPMAXU-EXCESS-AETLZ 0037900
0196 SPMAXL=0.9*FCL(SCIL)-SMLZ 0038000
0197 840 SMLZ=SMUZ+PERCL-AETLZ+EXCESS 0038100
0198 GC TC 56C 0038200
0199 C*** CALCULATE EVAPORATION FROM EARE SOIL SURFACE(SEVAP) FOR MONTHS 0038300
0200 C*** OCTOBER THROUGH MARCH OR WHEN THE DISPCAL AREA IS FALLOW 0038400
0201 860 AETUZ=0.0 0038500
0202 AETLZ=0.0 0038600
0203 IF(PACK-GT-C.0) GO TC 920 0038700
0204 IF(SMUZ.GE.FCU(SCIL)) EC=C.0 0038800
0205 0038900

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FORTAN IV C LEVEL 21

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0191 IF(SMUZ-LT.(FCU(SOIL)-U(SCIL))) GO TO 880 00393000
0192 C*** CALCULATE STAGE 1 SOIL EVAPCRATICN 00391000
0193 UZEVAP=SMUZ-(FCU(SOIL)-PETBS) 00392000
0194 IF(UZEVAP.LT.0.0) UZEVAP=0.0 00393000
0195 EC=EC+UZEVAP 00394000
0196 T=T+0 00395000
0197 IF(EG-GT.U(SCIL)) GO TO 880 00396000
0198 GO TO 900 00397000
0199 C*** CALCULATE STAGE 2 SOIL EVAPCRATICN 00398000
0200 T=T+1 00399000
0201 UZEVAP=C(SOIL)*(T**0.5)-(C(SOIL))*((T-1)**0.5) 00400000
0202 IF(UZEVAP.GT.(PETBS-IAET)) UZEVAP=PETBS-IAET 00401000
0203 IF(UZEVAP.LT.0.0) UZEVAP=0.0 00402000
0204 IF(SMUZ-PWPUZ(SOIL).LT.UZEVAP) UZEVAP=SMUZ-PWPUZ(SOIL) 00403000
0205 GO TO 540 00404000
0206 920 UZEVAP=0.0 00405000
0207 540 SMUZ=SMUZ-UZEVAP+PERC-EXCESS 00406000
0208 IF(SMUZ.LE.PWPUZ(SOIL)) SMUZ=PWPUZ(SOIL) 00407000
0209 SMUZ=SMUZ+PERC+EXCESS 00408000
0210 IF(SMUZ.LE.PWPUZ(SOIL)) SMUZ=PWPUZ(SOIL) 00409000
0211 DPERC=SMUZ-C.5*FCL(SOIL) 00410000
0212 IF(DPERC.LT.0.0) DPERC=0.0 00411000
0213 IF(SMUZ.GT.C.5*FCL(SOIL)) SMUZ=0.9*FCL(SOIL) 00412000
0214 AETUZ=AETUZ+UZEVAP 00413000
0215 SM IS SOIL THE MOISTURE IN THE GROWING ZONE, INCHES 00414000
0216 SM=SMUZ+SMUZ 00415000
0217 IAET=IA+IAACO 00416000
0218 C 00417000
0219 C 00418000
0220 C 00419000
0221 C 00420000
0222 T1 IS THE PREVIOUS DAY'S AVERAGE TEMPERATURE FAHRENHEIT DEGREES. 00421000
0223 T2 IS THE AVERAGE TEMPERATURE OF THE DAY TWO DAYS PRIOR OF TODAY 00422000
0224 T1=TAVG(T1)+T1+T2 00423000
0225 FROZE=TAVG(T1)+T1 00424000
0226 T1=TAVG(T1) 00425000
0227 IF(FROZE.LT.64.0) FROZE=1 00426000
0228 IF(T1.AVED.GT.114.0) FROZE=0 00427000
0229 WHEN FROZE EQUALS 1 THE SOIL IS CONSIDERED TO BE FROZEN. IT IS 00428000
0230 T1=AVG(T1)+T1+T2 00429000
0231 FROZE=1 00430000
0232 IF(FROZE.EQ.1) GO TO 580 00431000
0233 SMUZ IS THE SOIL MOISTURE IN THE TOP 12 INCHES OVER THE DISPOSAL 00432000
0234 AREA, FCU(SOIL) IS THE FIELD 00433000
0235 CAPACITY TO UTILIZE PRECIPITATION CN THE DISPOSAL AREA. WE WILL 00434000
0236 IRRIGATE SHOULD THE SOIL MOISTURE BE GREATER THAN THE PERCENTAGE 00435000
0237 CAPACITY SPECIFIED BY THE VARIABLE PSMN. 00436000
0238 IF(SMUZ.GT.PSMN*FCU(SOIL)) GO TO 580 00437000
0239 ICISCA=ICISCA+1 00438000
0240 DISVOL=DSRATE*DSAREA 00439000
0241 IF(PCNVOL.LE.DISVOL) GO TO 580 00440000
0242 C*** IF THE PCNVOL VOLUME IS LESS THAN THE VOLUME REQUIRED FOR ONE FULL 00441000
0243 DAY OF IRRIGATION, IT WILL BE ASSUMED THAT NO IRRIGATION WILL 00442000
0244 OCCUR CN THAT DAY. 00443000
0245 PCNVOL=PCNVOL-DISVOL 00444000
0246 GO TO 1000 00445000
0247 980 C1SVOL=0.0 00446000
0248 1000 CONTINUE 00447000

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FORTAN IV G LEVEL 21

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C
C
C*** UPDATE DISPOSAL DAY ACCOUNT
IF(DISVCL.GT.0.0) DSCAY=DSCAY+1
C
C
C
C
C*** CALCULATION CF FEEDLOT RUNOFF ***
C
C
C
C
C*** CALCULATE 3 DAY ANTECEDENT PCISTURE
A=P1+P2+P3
P1=P2
P2=P3
P3=PRECIP
IF (SNOW.GT.0.0.AND.FROZE.EQ.0) GO TO 102C
IF(PRECIP.LE.0.0) GC TC 1100
IF (FROZE.EQ.1) GO TO 108C
IF (SNOW.LE.0.5.AND.PRECIP.LE.0.5) GO TC 1100
C*** CALCULATE FEEDLOT RUNOFF USING 3 DAY ANTECEDENT MCISTURE
C*** CONDITIONS AND A MODIFICATION OF THE SCS METHOD
1020 A1=AM+PRECIP
PRECIP=PRECIP+SNOW
RC=97.0
IF (MONTH.LT.4.(CR.MONTH.GT.10) GO TO 1040
IF (AM.LT.0.75) RC=91.0
IF (A1.GT.GPCW.AND.PRECIP.GT.GROW) PRECIP=GROW
GO TO 106C
1040 IF (AM.LT.0.50) RC=91.0
IF (SNOW.GT.0.0) RC=97.0
IF (A1.GT.GCRP.AND.PRECIP.GT.DORM) PRECIP=DORM
CS=100.0/RC-10.0
RUNOFF=(PRECIP-0.2*CS)*2/((PRECIP+0.8*CS)
PUNOFF=PUNOFF+PRECIP-PRECIP+SNOW
SAC=C.0
IF (RUNOFF.GT.0.06) RUNOFF=RUNOFF-0.06
IF (PRECIP-0.2*CS.LT.0.0) GO TO 1100
GO TC 1120
1080 SNOW=SNOW+PRECIP
1100 RUNOFF=0.0
C
C
C
C
C*** CALCULATION CF SURFACE AREA AND DETERMINATION OF
SURFACE EVAPORATION FROM STORAGE FACILITY ***
C
C
C
C
C*** THE FOLLOWING CALCULATION EXPRESSES THE VOLUME CF WATER IN THE
STORAGE FACILITY IN CUBIC FEET.
1120 IF (PONVCL.LE.0.0) GC TC 1160
V=PONVCL*3630
C*** THE FOLLOWING CALCULATIONS DETERMINE THE SURFACE AREA OF THE
STORAGE FACILITY AS A FUNCTION OF STORAGE VOLUME. AREA IS IN
SQUARE FEET VOLUME IS IN CUBIC FEET. THE STORAGE FACILITY IS
SHAPE LIKE A FRUSTRUM CF A PYRAMIC. INPUT PARAMETERS TO SIZE
THE FACILITY ARE (L) OF THE BASE IN FEET, WIDTH OF THE BASE (W) IN
FEET AND SLOPE OF INSIDE EMBANKMENTS GIVEN AS A RATIO OF RUN TO
RISE(S). IT IS ASSUMED THE PCAD DOES NOT LEAK. INPUTS TO THE
STORAGE WILL BE RUNOFF AND PRECIPITATION. LOSSES FROM IT INCLUDE
EVAPORATION AND DISPOSAL VOLUME. B2 IS THE SURFACE AREA OF THE

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FCFTRAN IV 6 LEVEL 21

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C*** LIQUID IN SQUARE FEET.
1140 VC=AL*HAPRX+AL*HAPRX**2+AL*HAPRX**3
    CV=V-VC
    CVM=AL*AL*HAPRX+AL*HAPRX**2
    H=HAPRX+DV/CDCH
    IF(ABS(H-HAPRX).LT.0.1) GC TO 1160
    FAPRX=H
    GC TO 1140
1160 IF (H-GT-HMAX) H=HMAX
    R2=(L+2.*S*H)*(L+2.*S*H)
    IF(FROZE.EQ.1) LAKEVP=0.0
    LKEVPT=LKEVPT+LAKEVP
    SEVAP=R2*(LAKEVP/12)
    SEVAP IS THE VOLUME OF WATER EXTRACTED FROM THE STORAGE FACILITY
C***
C*** BY FREE SURFACE EVAPORATION.
    IF((SEVAP/3630).GT-PCNVCL) SEVAP=PCNVCL*3630
    PCNVCL=PCNVCL-(SEVAP/3630)
    IF(PCNVCL.LE.0.0) PCNVCL=0.0
C
C*** THE VOLUMES OF CALCULATED RUNOFF FROM THE FEEDLOT AND
C*** PRECIPITATION FALLING ON THE FACILITY ARE ADDED TO THE VOLUME
C*** OF WATER IN THE STORAGE FACILITY(ACRE-IN).
    PCNVCL=PCNVCL+(RUNOFF*LTAREA)*(PRECIP*PSAREA)
C
C*** THE VOLUME OF WATER REMAINING AT THE END OF THE DAY IS EXPRESSED
C*** IN ACRE-IN.
C
C
C*** THE FOLLOWING STATEMENTS DETERMINE WHETHER THE STORAGE FACILITY
C*** HAS OVERFLOWED AND IF SO, THE QUANTITY DISCHARGED
    DSCRG=0.0
    IF(PCNVCL-VCLMAX) 1220,1220,1180
    DSCRG=PCNVCL-VOLMAX
    PCNVCL=VCLMAX
1180 WRITE(6,1200) A,NO,YEAR,DSCRG
1200 FORMAT(20X,12,'/',12,'/',12,' ' ~ DISCHARGE OF',F10.2,' ACRE-IN')
C
C
1220 SMACCT(NM,2)=SMACCT(NM,2)+PRECIP
    SMACCT(NM,3)=SMACCT(NM,3)+DISVOL/DSAREA
    SMACCT(NM,4)=SMACCT(NM,4)+IA
    SMACCT(NM,5)=SMACCT(NM,5)+RNGF
    SMACCT(NM,6)=SMACCT(NM,6)+DPERC
    SMACCT(NM,7)=SMACCT(NM,7)+AETLZ+AETLZ+SNCVAP
    SMACCT(NM,8)=SMACCT(NM,8)+SM-SMPD
    SMPD=SM
    PDACCT(NM,3)=PDACCT(NM,3)+RLNCF*LYAREA/DSAREA
    PDACCT(NM,6)=PDACCT(NM,6)+SEVAP/(3630*DSAREA)
    PDACCT(NM,7)=PDACCT(NM,7)+DSCRG/DSAREA
    PDACCT(NM,8)=PDACCT(NM,8)+(PCNVOL-PDVCL)/DSAREA
    PCVCL=PCNVCL
    IF(PCNVCL.GT-MAXVOL) MAXVOL=PCNVCL
C
1240 CCNTINUE
C
C
C ***** EXIT DAILY LCOP *****

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FORTRAN IV G LEVEL 21      MAIN      DATE = 76119      00/08/30
0332      1480 FORMAT('0',10X,'ESTIMATED LAKE EVAPORATION, INCHES =',F6.2)      00620000
0333      1500 CCNTINUE      00621000
      C      ***** EXIT YEARLY LOOP *****      00622000
      C      1520 STOP      00623000
      C      END      00624000
      0334      00625000
      0335      00626000

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STATION: TOPEKA, KANSAS 1949 TO 1973

SIZE OF CRITICAL EVENT 6.10

FEEDCT AREA 40.00ACRES

PCNO VARIABLES:

(A) BASE DIMENSION-- 400.00 FEET BY 200.00 FEET
 (B) SIDE SLOPE-- RUN: RISE = 3.0 : 1
 (C) MAXIMUM DEPTH-- 9.00 FEET
 (D) MAXIMUM PCNO VOLUME-- 240.92 ACRE-INCHES
 (E) DIRECT RECEIVING AREA (FOR PRECIPITATION) -- 2.65 ACRES

DISPCAL AREA VARIABLES:

(A) DISPOSAL AREA-- 80.00 ACRES
 (B) CROP-- SORGHUM
 (C) SOIL TYPE-- 3 (SCS) SOIL TYPES
 (D) DISPOSAL RATE-- 6.61 INCHES/DAY ON DISPOSAL DAYS
 (E) IRRIGATION MANAGEMENT-- IRRIGATION BELOW 0.90 FIELD CAPACITY

***** ANNUAL SUMMARY *****

WATER ACCOUNT FOR STORAGE FACILITY (IN INCHES OVER DISPOSAL AREA) - 1956

MONTH	INFLOWS			OUTFLOWS			CHANGE IN VCL
	PRECIPITATION	FEEDLOT RUNOFF	NO. DISPOSAL DAYS	DISPOSAL VOL. C.C.	SURFACE EVAP.	DISCHARGE	
JAN.	0.02	0.0	0.	0.0	0.01	0.0	0.02
FEB.	0.02	0.52	1.	0.61	0.02	0.0	-0.09
MAR.	0.01	0.03	0.	0.0	0.07	0.0	-0.03
APR.	0.13	0.74	0.	0.0	0.12	0.0	0.75
MAY	0.08	0.14	1.	0.61	0.17	0.0	-0.56
JUNE	0.18	0.52	0.	0.0	0.20	0.0	0.90
JULY	0.15	0.87	3.	1.83	0.22	0.0	-1.02
AUG.	0.08	0.20	1.	0.61	0.10	0.0	-0.35
SEPT.	0.03	0.16	0.	0.0	0.11	0.0	0.08
OCT.	0.04	0.0	0.	0.0	0.09	0.0	-0.05
NOV.	0.06	0.45	0.	0.0	0.04	0.0	0.47
DEC.	0.03	0.00	0.	0.0	0.01	0.0	0.01
TOT.	0.84	4.11	6.	3.66	1.14	0.0	0.15

WATER BALANCE (INCHES) IN THE DISPOSAL AREA - 1956

MONTH	INPUTS			OUTPUTS			CHANGE IN SM
	PRECIPITATION	IRRIGATION	INTERCEPTION	SURFACE RUNOFF	PERCOLATION	AET	
JAN.	0.75	0.0	0.12	0.0	0.0	0.24	-0.12
FEB.	0.70	0.61	0.34	0.01	0.0	0.18	1.29
MAR.	0.42	0.0	0.29	0.00	0.0	0.19	-0.06
APR.	3.94	0.0	0.51	1.58	0.0	1.06	0.79
MAY	2.36	0.61	0.90	0.33	0.0	1.57	0.17
JUNE	5.22	0.0	0.82	2.27	0.0	1.66	0.57
JULY	4.46	1.83	1.26	1.38	0.0	5.72	-2.09
AUG.	2.35	0.61	0.70	0.09	0.0	4.15	-1.58
SEPT.	1.02	0.0	0.10	0.00	0.0	1.11	-0.19
OCT.	1.33	0.0	0.40	0.0	0.0	0.29	0.64
NOV.	1.96	0.0	0.34	0.06	0.0	0.64	0.91
DEC.	0.76	0.0	0.48	0.0	0.0	0.18	0.10
TOT.	25.35	3.66	6.27	5.73	0.0	17.00	0.01

PERCENT OF WASTEWATER CONTROLLED= 100.00

POTENTIAL DISPOSAL DAYS= 155

PACK ON DECEMBER 31 = C.C

CHANGE IN SNOW STORAGE= 0.0

INFLOWS-OUTFLOWS-CHANGE IN SNOW STORAGE=CHANGE IN SOIL MOISTURE

PERCENT OF MAXIMUM FEED VOLUME REQUIRED = 60.21

ESTIMATED LAKE EVAPORATION, INCHES = 45.85

**** ANNUAL SUMMARY ****
 6/ 2/68 - DISCHARGE CF 57.66 ACRE-IN
 8/ 9/68 - DISCHARGE CF 10.12 ACRE-IN
 8/10/68 - DISCHARGE CF 37.69 ACRE-IN

WATER ACCOUNT FOR STORAGE FACILITY (IN INCHES OVER DISPOSAL AREA) - 1968

INFLOWS					OUTFLOWS				
MCNTH	PRECIPITATION	FEEDLOT RUNOFF	NO. DISPOSAL DAYS	DISPOSAL VOL.	SURFACE EVAP.	DISCHARGE	CHANGE IN VOL.		
JAN.	C.03	0.21	C.	C.0	0.01	0.0	0.23		
FEB.	C.02	0.0	0.	0.0	0.02	0.0	0.00		
MAR.	C.02	0.09	C.	0.0	0.09	0.0	0.02		
APR.	0.14	0.52	3.	1.83	0.14	0.0	-1.31		
MAY	C.11	0.12	1.	0.61	0.14	0.0	-0.52		
JUNE	C.11	0.61	1.	0.61	0.20	0.0	-0.09		
JULY	C.34	2.06	1.	0.61	0.20	C.0	2.50		
AUG.	C.24	1.58	5.	3.05	0.20	1.32	-2.34		
SEPT.	0.08	0.40	1.	0.61	0.14	0.0	-0.18		
OCT.	C.14	1.07	0.	0.0	0.10	C.0	1.11		
NOV.	C.05	0.21	1.	0.61	0.04	0.0	-0.39		
DEC.	0.07	0.0	0.	0.0	0.01	C.0	0.06		
TOT.	1.34	8.26	13.	7.93	1.28	1.32	-0.92		

WATER BALANCE (INCHES) IN THE DISPOSAL AREA - 1968

INFLWS					OUTFLWS				
MCNTH	PRECIPITATION	IRRIGATION	INTERCEPTION	SURFACE RUNOFF	PERCOLATION	AET	CHANGE IN SP		
JAN.	0.89	0.0	0.26	0.13	C.0	0.10	0.73		
FEB.	0.56	0.0	0.27	0.03	C.09	0.10	0.05		
MAR.	0.46	0.0	C.27	0.02	0.0	0.14	C.03		
APR.	4.20	1.83	1.03	1.16	1.40	2.73	-0.29		
MAY	3.37	0.61	0.92	0.39	0.07	2.29	0.31		
JUNE	3.18	0.61	C.43	1.55	0.53	1.62	-0.34		
JULY	10.17	0.61	1.08	4.87	C.0	5.58	-0.74		
AUG.	7.40	3.05	1.31	3.67	0.0	6.22	-0.74		
SEPT.	2.50	0.61	0.62	0.48	0.0	3.54	-1.53		
OCT.	4.19	0.0	0.58	2.05	0.0	0.93	0.63		
NOV.	1.56	0.61	C.50	0.38	0.0	1.13	0.16		
DEC.	2.10	0.0	0.30	0.67	0.0	0.50	0.41		
TOT.	40.58	7.93	7.57	15.40	2.09	24.88	-1.32		

PERCENT OF WASTEWATER CONTROLLED= 86.27

POTENTIAL DISPOSAL DAYS= 60

PACK ON DECEMBER 31 = C.22 CHANGE IN SNOW STORAGE=-0.10

INFLOWS-OUTPUTS-CHANGE IN SNOW STORAGE=CHANGE IN SOIL MOISTURE

PERCENT OF MAXIMUM PCND VOLUME REQUIRED = 100.00

ESTIMATED LAKE EVAPORATION, INCHES = 47.52

***** ANNUAL SUMMARY *****

WATER ACCOUNT FOR STORAGE FACILITY (IN INCHES OVER DISPOSAL AREA) - 1970

MONTH	INFLOWS				OUTFLOWS				CHANGE IN VOL.
	PRECIPITATION	FEEDLOT RUNOFF	NO. DISPOSAL DAYS	DISPOSAL VOL.	SURFACE EVAP.	DISCHARGE			
JAN.	0.01	0.22	1.	0.61	0.01	0.0			-0.39
FEB.	0.01	0.0	0.	0.0	0.02	0.0			-0.01
MAR.	0.03	0.02	0.	0.0	0.03	0.0			0.03
APR.	0.12	1.00	1.	0.61	0.11	0.0			0.40
MAY	0.18	0.99	1.	0.61	0.16	0.0			0.40
JUNE	0.19	1.42	0.	0.0	0.22	0.0			1.40
JULY	0.05	0.08	3.	1.83	0.20	0.0			-1.90
AUG.	0.03	0.00	0.	0.0	0.17	0.0			-0.14
SEPT.	0.25	1.91	2.	1.22	0.14	0.0			0.80
OCT.	0.08	0.25	0.	0.0	0.09	0.0			0.24
NOV.	0.04	0.0	0.	0.0	0.04	0.0			-0.00
DEC.	0.05	0.0	0.	0.0	0.01	0.0			0.04
TOT.	1.05	5.89	8.	4.88	1.20	0.0			0.86

WATER BALANCE (INCHES) IN THE DISPOSAL AREA - 1970

MONTH	INPUTS				OUTPUTS				CHANGE IN SP
	PRECIPITATION	IRRIGATION	INTERCEPTION	SURFACE RUNOFF	PERCOLATION	AET			
JAN.	0.19	0.61	0.26	0.0	0.0	0.16			0.58
FEB.	0.34	0.0	0.21	0.00	0.0	0.05			0.07
MAR.	1.03	0.0	0.38	0.18	0.0	0.19			0.27
APR.	3.49	0.61	0.57	0.76	0.0	2.12			0.64
MAY	5.46	0.61	1.11	1.35	0.0	2.67			0.54
JUNE	5.82	0.0	0.88	2.49	0.0	1.57			0.88
JULY	1.39	1.83	0.63	0.05	0.0	5.62			-3.08
AUG.	0.83	0.0	0.44	0.0	0.0	2.48			-2.09
SEPT.	7.70	1.22	1.24	2.70	0.0	1.93			2.44
OCT.	2.49	0.0	0.70	0.51	0.0	0.85			1.04
NOV.	1.23	0.0	0.47	0.12	0.0	0.33			0.31
DEC.	1.65	0.0	0.43	0.30	0.0	0.04			0.85
TOT.	31.62	4.88	7.33	8.47	0.0	18.02			2.84

PERCENT OF WASTEWATER CONTROLLED= 100.00

POTENTIAL DISPOSAL DAYS= 99

PACK ON DECEMBER 31 = 0.03 CHANGE IN SNOW STORAGE=-0.17

INPUTS-CUTLETS-CHANGE IN SNOW STORAGE=CHANGE IN SCIL MOISTURE

PERCENT OF MAXIMUM POND VOLUME REQUIRED = 76.67

ESTIMATED LAKE EVAPORATION, INCHES = 48.05

A CONTINUOUS WATERSHED MODEL FOR EVALUATION AND
DESIGN OF FEEDLOT RUNOFF CONTROL SYSTEMS

by

Theodore A. Bean

B.S., Kansas State University, 1974

AN ABSTRACT OF A MASTER'S THESIS

submitted in partial fulfillment of the
requirements of the degree

MASTER OF SCIENCE

Department of Civil Engineering

KANSAS STATE UNIVERSITY
Manhattan, Kansas

1976

ABSTRACT

A continuous watershed model has been developed for the purpose of evaluation and design of feedlot runoff control systems. This computerized model programmed in the standard Fortran IV language basically consists of three components; 1) functions to generate runoff from the feedlot surface, 2) functions which simulate water movement and soil-moisture conditions within an agricultural disposal area, and 3) functions to monitor inflows, outflows, and water-level fluctuations of the storage facility. The functions used in this model are based on physical parameters which will enable use in any geographical province in the U.S. Required inputs consist of meteorological data, soil, and crop parameters which are readily available. The model may be used by designers, feedlot managers, and regulatory agencies for the evaluation and design of feedlot control systems.